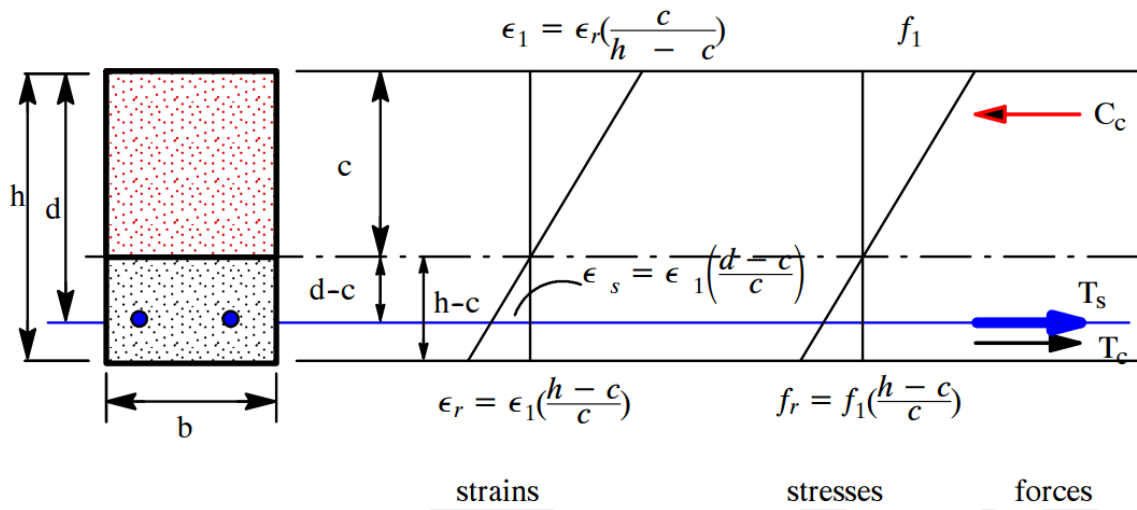


## Reinforced Concrete Beam - Uncracked

### 1. Assumptions:

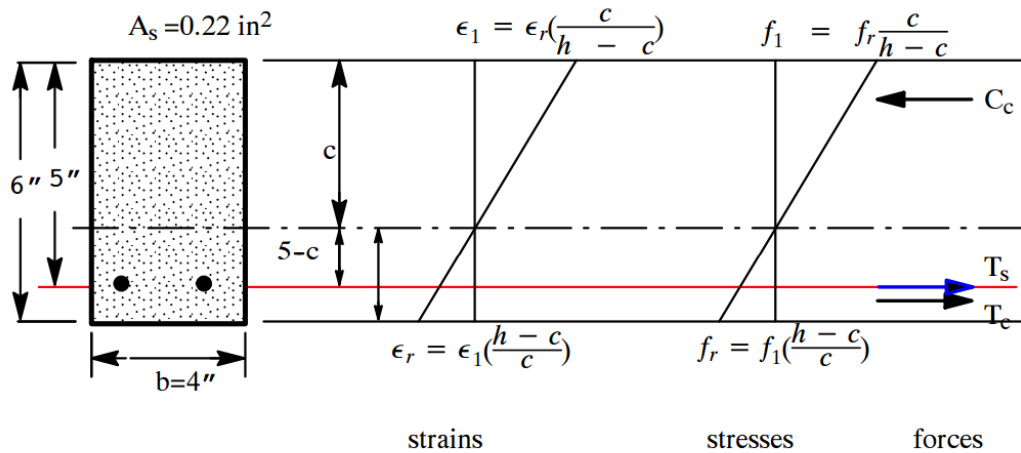
- a. Strains vary linearly with distance from N.A.
- b. Linear stress-strain relationship;
- c. Strain compatibility between steel and concrete  $\epsilon = \epsilon_s = \epsilon_c$

### 2. Reinforced concrete beam. Before cracking



### Example 1. Calculate Cracking Moment ( $M_{cr}$ )

Calculate the moment of the section shown below when maximum tensile stress in concrete is equal to  $f_r$  (Cracking Moment)



#### Given Material Properties

$$f'_c = 3200 \text{ psi}$$

$$f_r = 500 \text{ psi} = 0.5 \text{ ksi}$$

$$E_c = 57,000 \sqrt{3200} = 3,220,000 \text{ psi} = 3,220 \text{ ksi}$$

#### Solution

$$\rho = \frac{A_s}{bd} = \frac{0.22(\text{in}^2)}{4(\text{in}) \times 5(\text{in})} = 0.011$$

$$\frac{E_s}{E_c} = n = \frac{29,000 \text{ (ksi)}}{3,220 \text{ (ksi)}} = 9.01 \approx 9$$

$$\frac{h}{d} = \frac{6}{5} = 1.2$$

$$\frac{c}{d} = \frac{2\rho(n - 1) + \left(\frac{h}{d}\right)^2}{2\rho(n - 1) + 2\left(\frac{h}{d}\right)} = \frac{2 \times 0.011 \times 8 + (1.2)^2}{2 \times 0.011 \times 8 + 2 \times (1.2)} = 0.627$$

$$c = 0.627d = 0.627 \times 5(\text{in}) = 3.14 \text{ inches}$$

$$f_1 = f_r \left( \frac{c}{h - c} \right) = 0.5 \times \frac{3.14}{6 - 3.14} = 0.549 \text{ ksi}$$

Having  $f_1$ , we can easily calculate all forces:

$$C_c = \frac{1}{2} f_1 c b = \frac{1}{2} \times 0.549(\text{ksi}) \times 3.14(\text{in}) \times 4(\text{in}) = 3.45 \text{ kip}$$

$$\begin{aligned} T_s &= f_1 \left( \frac{d - c}{c} \right) (n - 1) A_s \\ &= 0.549(\text{ksi}) \times \frac{5(\text{in}) - 3.14(\text{in})}{3.14(\text{in})} \times (9 - 1) \times 0.22(\text{in}^2) = 0.57 \text{ kips} \end{aligned}$$

$$\begin{aligned} T_c &= \frac{1}{2} f_1 \left( \frac{h - c}{c} \right) (h - c) b \\ &= \frac{1}{2} \times 0.549(\text{ksi}) \times \frac{[6(\text{in}) - 3.14(\text{in})]^2}{3.14(\text{in})} \times 4(\text{in}) = 2.86 \text{ kips} \end{aligned}$$

Check equilibrium, does it satisfy  $C_c = T_s + T_c$  ?

$C_c = 0.57 + 2.86 = 3.43 \text{ kips} = C_c \text{ kips}$  ; the difference is due to rounding error associated with calculating "c"

Calculate moment about N.A. ( or any point on the cross section)

Force Kips	Moment Arm inches	Moment in-kips
$C_c = 3.45 \text{ kips}$	$\frac{2}{3}c = \frac{2}{3} \times 3.14 = 2.09$	$= 7.21$ ↗
$T_s = 0.57$	$d - c = 5 - 3.14 = 1.86$	$= 1.06$ ↗
$T_c = 2.86$	$\frac{2}{3}(h - c) = \frac{2}{3}(6 - 3.14) = 1.91$	$= 5.46$ ↗
		<u>        </u>
		Total M = 13.73 in-kips