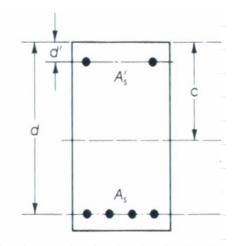
#### **Analysis of Doubly reinforced rectangular Beam**

Example: Doubly Reinforced Section

h=18 in. b=12 in.

#### Given:

$$f'_{c}$$
= 4000 psi  $f_{y}$  = 60 ksi  
 $A'_{s}$  = 2 #5  $A_{s}$  = 4 #7  
 $d'$ = 2.5 in. d = 15.5 in



Calculate  $M_n$  for the section for the given compression steel.

Compute the reinforcement coefficients, the area of the bars #7 (0.6 in<sup>2</sup>) and #5 (0.31 in<sup>2</sup>)

$$A_{s} = 4(0.6 \text{ in}^{2}) = 2.4 \text{ in}^{2}$$

$$A'_{s} = 2(0.31 \text{ in}^{2}) = 0.62 \text{ in}^{2}$$

$$\rho = \frac{A_{s}}{bd} = \frac{2.4 \text{ in}^{2}}{(12 \text{ in.})(15.5 \text{ in.})} = 0.0129$$

$$\rho' = \frac{A'_{s}}{bd} = \frac{0.62 \text{ in}^{2}}{(12 \text{ in.})(15.5 \text{ in.})} = 0.0033$$

# Compute the effective reinforcement ratio and minimum $\rho$

$$\rho_{\it eff} = \rho - \rho' = 0.0129 - 0.0033 = 0.00957$$

$$\rho = \frac{200}{f_{\rm y}} = \frac{200}{60000} = 0.00333$$

or 
$$\frac{3\sqrt{f_c}}{f_y} = \frac{3\sqrt{4000}}{60000} = 0.00316$$

$$\rho \ge \rho_{\min} \Rightarrow 0.0129 \ge 0.00333 \text{ OK!}$$

Compute the effective reinforcement ratio and minimum  $\rho$ 

$$(\rho - \rho') \ge \left(\frac{\beta_1(0.85 f_{c}')d'}{d f_{y}}\right) \left(\frac{87}{87 - f_{y}}\right)$$

$$\geq \left(\frac{0.85(0.85(4 \text{ ksi}))(2.5 \text{ in.})}{60 \text{ ksi}(15.5 \text{ in.})}\right) \left(\frac{87}{87-60}\right) = 0.0398$$

0.00957 ≥ 0.0398 Compression steel has not yielded.

## Instead of iterating the equation use the quadratic method

$$0.85 f_c b \beta_1 c^2 + (A'_s E_s \varepsilon_{cu} - A_s f_y) c - A'_s E_s \varepsilon_{cu} d' = 0$$

$$0.85 (4 \text{ ksi}) (12 \text{ in.}) (0.85) c^2 +$$

$$+ \left[ ((0.62 \text{ in}^2) (29000 \text{ ksi}) (0.003) - (2.4 \text{ in}^2) (60 \text{ ksi})) \right] c$$

$$- (0.62 \text{ in}^2) (29000 \text{ ksi}) (0.003) (2.5 \text{ in.}) = 0$$

$$34.68 c^2 - 90.06 c - 134.85 = 0$$

$$c^2 - 2.5969 c - 3.8884 = 0$$

## Solve using the quadratic formula

$$c^2 - 2.5969c - 3.8884 = 0$$

$$c = \frac{2.5969 \pm \sqrt{(-2.5969)^2 - 4(-3.8884)}}{2}$$

$$c = 3.6595 \text{ in.}$$

$$f_{\rm s}' = \left(1 - \frac{d'}{c}\right) E_{\rm s} \varepsilon_{\rm cu} = \left(1 - \frac{2.5 \text{ in.}}{3.659 \text{ in.}}\right) 87 \text{ ksi}$$
  
= 27.565 ksi

Check the tension steel.

$$\varepsilon_{\rm s} = \left(\frac{15.5 \text{ in.} - 3.659 \text{ in.}}{3.659 \text{ in.}}\right) 0.003 = 0.00971 \ge 0.00207$$

#### Check to see if c works

$$c = \frac{A_{s}f_{y} - A'_{s}f'_{s}}{0.85f_{c}\beta_{1}b} = \frac{(2.4 \text{ in}^{2})(60 \text{ ksi}) - (0.62 \text{ in}^{2})(27.565 \text{ ksi})}{0.85(4 \text{ ksi})(0.85)(12 \text{ in.})}$$

$$c = 3.659 \text{ in.}$$

### Compute the moment capacity of the beam

$$M_{n} = \left(A_{s}f_{y} - A_{s}'f_{s}'\right)\left(d - \frac{a}{2}\right) + A_{s}'f_{s}'(d - d')$$

$$= \left(\frac{(2.4 \text{ in}^{2})(60 \text{ ksi})}{-(0.62 \text{ in}^{2})(27.565 \text{ ksi})}\right)\left(15.5 \text{ in.} - \frac{0.85(3.659 \text{ in.})}{2}\right)$$

$$+ \left(0.62 \text{ in}^{2}\right)(27.565 \text{ ksi})(15.5 \text{ in.} - 2.5 \text{ in.})$$

$$= 1991.9 \text{ k-in.} \Rightarrow 166 \text{ k-ft}$$

## The resulting ultimate moment is

$$M_{\rm u} = \phi M_{\rm u} = 0.9 (166 \text{ k-ft})$$
  
= 149.4 k-ft